

SECTION 610 – SEWER AND WATER CONSTRUCTION

SECTION 610 - CONSTRUCTION OF SANITARY AND STORM SEWERS AND LATERALS

New sewer and water construction must be completed prior to the general pavement removal operation. Preparation of the Right-of-Way, saw-cutting and removals shall be in accordance with Section 620 of these specifications.

Inlet shapes are to be altered in accordance with the iron to be used. Masonry shims on sewer structures must fully cover the masonry below. The pitch across the frame should be set to match the concrete curb and gutter cross-section.

Trench work shall not begin so far in advance of rough grading work that the gravel backfill will require more than a 6 week maintenance interval. The Contractor will be required to maintain the trench area during the interval as incidental to the work, by keeping it to grade and spreading calcium chloride, if necessary, for dust control. Aggregate slurry backfill must be used if excavation is to be paved over in less than 10 calendar days or as indicated on the plans. All sewer structure work shall be done in accordance with the Standard Specs and the City Specs.

Excavation and backfilling relating to utility installation is incidental to the utility bid item.

610.1.01 - EXCAVATION

A. GENERAL

Excavation required for this work for the most part is unclassified. Complete all excavation regardless of the type of materials encountered. The Contractor shall make their own estimate of the kind and extent of the various materials which will be encountered in the excavation, including the presence or absence of water. The surface type as shown on the plans is presented only as a guide for the Contractor and does not guarantee the type or depth of material beneath the surface course. No additional compensation will be made for differing surface materials. No additional compensation will be made for any rails, ties, or other unknown structures and objects that may be encountered. The Contractor may make written requests to the Engineer for exceptions to this rule, however the Engineer is under no obligation to approve exceptions.

The Contractor shall expose both ends of spot relays before commencing any pipe laying so that line and grade may be adjusted.

If a concrete cradle, cap, or envelope exists on any sewer to be removed and it is not noted on the plans, payment for the removal will be at 80% of the rate for rock excavation shown in the Schedule of Fixed Extras. The fixed extra price will include

all additional costs including, but not limited to, any additional labor, material, time, equipment, excavation, backfill, shoring, bracing, pavement removal and replacement, fees, and trucking. Excavation by hand means the use of pneumatic hand tools. Mechanical excavation requires the use of special attachments on excavators.

These prices will be used for removals up to 40 linear feet. If the concrete cradle, cap or envelope extends for greater lengths, a price for the remainder of the removal shall be mutually agreed upon with the Engineer before the Contractor continues. When computing the volume removed, no subtraction will be made for the cross-sectional area of the existing pipe. If the concrete cradle, cap, or envelope are shown on the plans, the price of removal should be included in the price for the sewer relay.

The spot relay shall be in a straight line grade from the downstream end of the existing pipe to the upstream end. Additional piping removed and replaced in order to provide positive drainage to the downstream end will be paid at the contract price for a longer spot relay or at the relay contract per linear foot price when the relay exceeds 25.5 linear feet. The pipes coming into and going out of the spot relay shall be checked with a hand level to make sure that they do not back pitch. Additional pipe should be removed to eliminate back-pitch and will be incidental to the spot relay/repair.

The location, size, and elevation of all underground structures shown on the plans have been located to a reasonable degree of accuracy, but the City does not guarantee their exact location and data or the location and data of others not shown. Concrete support columns shall be placed on all sewers where shown on the plans and at all other locations not shown where a utility in a rigid conduit is discovered to pass beneath the new sewer by less than 12 inches. Concrete support columns shall be formed. Bank pouring of concrete support columns will not be permitted. The costs of these supports will be considered incidental to the contract.

Bridging, where needed or where directed to be placed, shall be provided and installed by the Contractor at no additional cost to the City.

If any damage occurs to an underground facility, or the damage is found to exist, such that the protective coating of an electrical line is penetrated or gases or liquids are escaping from a broken line which endangers life, health, or property, the Contractor shall immediately call "911" to report the damage location. This call shall be made prior to contacting the utility involved.

1. SALVAGED MATERIALS

See Section 620 of the specifications for materials to be salvaged.

610.1.02 - LAYING OF PIPE

A. BEDDING, COVER, FOUNDATION AND BACKFILL MATERIAL

All sewer pipe shall be laid in a Standard Section, Class "C" bedding of crushed limestone conforming to File No. 3 and Table 32 or Table 38, as applicable to size and type of pipe material, of the Standard Specs, with modifications as specified in Section 3.2.6(i) for PVC pipe of the Standard Specs, unless otherwise noted on the plans. Cover material for pipe shall be the same as that specified for bedding. Risers which do not require a concrete envelope shall be bedded (i.e. surrounded by bedding material) all the way to the top of the riser, and special care shall be taken to "tuck" the bedding tightly around the entire lateral assembly for all laterals to prevent future settling.

Backfill used on this contract for sanitary and storm sewer work shall be either mechanically compacted (unless directed to use flooding by the Engineer) crushed recycled concrete 1-1/4 inch dense meeting the gradation requirements for granular material as specified in Table 37 in Section 8.43.4 of the Standard Specs, or aggregate slurry backfill as specified in Section 8.43.8 of the Standard Specs. The backfill shall be consolidated by mechanical compaction of the trench backfill as specified in Section 2.6.14(b) of the Standard Specs unless otherwise specified by the Engineer.

Lumps of clay, loam, spoils (unless otherwise stated), garbage, organic material, or any other material the Engineer deems unsuitable are not allowed in the backfill, and the Engineer reserves the right to order the Contractor to remove such items from the trench before paving commences, at the Contractor's expense, should the pieces be deemed unreasonably large and/or numerous. Material resulting from incidents such as, but not limited to, trench wall collapses is NOT excluded from this rule. For instances where the Contractor does not remove unacceptable backfill when directed, they shall be charged a percentage of the price for the pipe over the lineal footage in which the fill is present, to be deducted from monies owed to the Contractor.

B. PIPE AND FITTINGS

Except for lateral reconnections, sizes and strength classifications of sanitary sewer pipe to be used in all locations are indicated on the plans.

All wyes and tees shall be moulded as a single piece only. No wyes or tees with glued and/or fused pieces will be accepted unless the Contractor is given the written permission of the Engineer.

PVC pipe shall conform to ASTM D-3034 SDR35, Type PSM, rubber gasket joints, or ASTM F-789 Type PS-46 for sizes 4 inches through 15 inches and F-679 (T-I)

12454 Type PSM rubber gasket joints for sizes 18 inches through 27 inches. Where PVC pressure pipe is called for on the plans, it shall conform to AWWA C900, Pressure Class 150 (PC150) DR18. Joints shall use elastomeric gaskets.

All concrete pipe to be used for storm sewer on this project shall be reinforced concrete pipe, ASTM C-76 or ASTM C-507, or of the class shown on the plans. Reinforced concrete horizontal elliptical pipe 18 inch or larger in diameter shall conform to ASTM C 507 and of the class as shown on the plans. All reinforced concrete pipe catch basin or inlet leads, regardless of size, shall be ASTM C-76 Class V. Pipe furnished under this classification as manufactured by American Concrete Pipe Co., Inc., Milwaukee and Green Bay, WI, Madison Concrete Pipe, Inc., Madison, WI, County Concrete Corp., Marathon, WI, or equal, shall meet the requirements set forth in ASTM C-76 with "B" or "C" wall for circular pipe and any additional requirements set forth herein and in Chapter 8.6.0 of the Standard Specs. Joints shall use rubber gaskets in all concrete pipes (including circular and elliptical) and concrete box culverts, unless otherwise approved by the Engineer.

Temporary repairs for storm sewers which are to be replaced before the completion of the project may be made with PVC class SDR35 pipe, and the joints may be made by any reasonable means to prevent leaking and backups before the future replacement, at the discretion of the Engineer. Permanent storm spot repairs which were not originally called for on the plans or in a change order may be made with PVC C900 pipe with antihydraulic mortar joints if the Contractor does not wish to use RCP, with the permission of the Engineer and at no additional cost to the City.

Joints for concrete storm sewer reducers and bends shall be submitted to the Engineer and approved prior to construction as part of the shop drawing review.

The Contractor shall bear all costs of testing and shall submit copies of these test reports to the Engineer prior to any pipe installation.

C. JOINTS BETWEEN DISSIMILAR PIPE MATERIALS

When field cutting and/or machining the pipe is necessary, use only tools and methods recommended by the pipe manufacturer and approved by the Engineer. Breaking and chipping the pipe with a wrench, pliers, or a hammer will not be allowed.

Connect dissimilar pipe materials by means of a non-shear flexible compression coupling as specified below for sanitary sewer and for storm sewer, or a concrete closure collar for storm sewers as directed by the Engineer. Install couplings in strict accordance with the manufacturer's recommendations.

Joints on sanitary sewers between dissimilar pipe shall be either a non-shear coupling as manufactured by DFW/HPI or shall be made with flexible mechanical

compression joint coupling conforming to ASTM C-594 Type B with stainless steel bands and shear ring conforming to ASTM A-167 as manufactured by Joints, Inc. (Calder) of Gardena, CA, Fernco Joint Sealer Co. of Ferndale, MI., or as manufactured by Gripper Gasket, LLC of Corona, CA., or equal, and in addition using a transitional bushing conforming to ASTM C-594 Type B when pipe with different outside diameters are to be connected.

If a connection is being made to an existing lined pipe, the connection shall be made with one of the above adaptors directly to the liner of the pipe. A connection to the host pipe will not be permitted.

Joints on storm sewers between dissimilar pipe may be made with either a non-shear flexible mechanical compression joint coupling with No. 305 stainless steel bands or, where this is not possible, a concrete closure collar as shown on the special detail with prior approval of the Engineer.

Use concrete closure collars only on nonflexible pipe storm sewers and when approved by the Engineer, and then only to make connections between dissimilar pipe when standard rubber gasketed joints, mortar, or flexible couplings are impractical.

Before the closure collars are poured, wash the pipe to remove all loose material and soil from the surface on which the concrete will be placed. Wet nonmetallic pipe thoroughly prior to pouring the collars. Wrap and securely fasten a light gauge of sheet metal or building felt around the pipe to insure that no concrete shall enter the line. Place reinforcement as needed. Make the entire collar in one pour using 3000 psi concrete and extend a minimum of 12 inches on each side of the joint. The minimum thickness around the outside diameter of the pipe shall be 6 inches. No collar shall be poured in water. After the collars are poured and have taken their initial set, cure by covering with well-moistened earth. Refer to a special detail drawing(s), if provided.

Payment for connecting dissimilar pipe materials with flexible couplings, non-shear couplings or, when approved by the Engineer, concrete closure collars shall be included in, and incidental to, the prices for pipe stated in the bid unless otherwise noted as a separate item.

D. ABANDONED SEWERS, DRAINS AND SEWER STRUCTURES

Where the plans call for a sewer to be abandoned, the Contractor has the option to either remove or abandon the sewer by bulkheading the ends and filling the pipe with concrete grout as specified in Sections 8.35.4 or 8.35.5 of the Standard Specs, or as directed or approved by the Engineer. Direction of the use of Elastizell PS120 which is not called for on the plans or included in a bid item description shall be paid as an extra cost to the Contractor per cubic yard of material installed.

Bulkheads shall be as specified in Chapter 3.2.25 of the Standard Specs. Where the plans call for removal, the Contractor shall remove the entire pipe including any concrete support and backfill with the material as specified. The cost of this work shall be incidental unless otherwise specified as a separate bid item.

Manholes shall be removed or abandoned where specified or shown on the plans. As with sewers, the Contractor has the option to remove structures which are identified for abandonment but must remove those identified to be removed. The removal shall include the base of the structure. Manhole caps shall be used in the abandonment of sanitary or storm sewer structures and shall be made to the satisfaction of the Engineer. Where a cap is used, the manhole shall be filled with slurry or other suitable materials at least 4 feet below the proposed finished grade. This work shall be incidental to the contract unless noted as a separate bid item.

E. INSULATION AROUND WATER MAINS & APPURTENANCES

The Contractor shall provide and install extruded polystyrene rigid insulation with a minimum of 25 PSI compressive strength, two layers of 2 inch thick by 6 feet long by the width of the trench, between storm sewer and structure installations and all water mains and services at all locations shown on the plans, and at any other location where a water main or service is exposed. Insulation shall also be placed above storm sewers where they cross under water mains and services with less than 12 inches of separation, or as directed by the Engineer. Include costs with the price of the storm sewer relay, repair, extension, or structure, or water main relay, repair, or extension. If the vertical separation is greater than 12 inches and the service or main has a minimum of 6 feet of cover, the insulation need not be installed at locations which are not shown on the plans. Insulation shall be incidental to the work unless noted as a separate bid item.

F. CONNECTION TO EXISTING STRUCTURES

Where the sewer relay, repair, or extension begins or ends with a connection to an existing structure, the Contractor shall remove existing pipe and masonry from the structure as needed to make the new connection. The Contractor shall install an Engineer approved flexible water tight pipe-to-MH seal ("boot") for all sanitary sewer and other flexible pipe connections. For rigid pipe storm sewer connections, the Contractor may mortar the pipe into place. The structure's paved invert shall also be modified and rebuilt as needed. All costs shall be included with the price of the sewer unless noted as a separate bid item.

G. CONCRETE BEAMS & CRADLES

Concrete beams/cradles shall be constructed or placed where shown in the plans or as directed by the Engineer. Concrete beams shall conform to File No. 2 of the

Standard Specs. Concrete cradles shall conform to 3.2.6 (c) of the Standard Specifications except as modified herein.

Concrete cradles shall be monolithically poured and must be constructed using wood forms or other Engineer approved forming material. Concrete cradles shall be poured and permitted to cure for a minimum of 24 hours prior to setting the pipe on the cradle. The pipe shall not be placed on blocks or hardwood prior to the the cradle being formed & cured and these materials shall not be incorporated in any way into the cradle.

Concrete cradles Concrete used for concrete cradles shall conform to Section 620 of these specifications.

H. RESTRAINTS FOR ENDWALL SECTIONS AND BOX CULVERTS

Where a storm sewer terminates into an endwall section, the last three sections of pipe into the endwall section, including the joint to the endwall section, shall be mechanically restrained. Submit the type and number of proposed endwall restraints for each joint to the Engineer for review and approval.

Box culverts shall have the last three joint sections restrained at an outfall, similar to an endwall section. Box culverts shall also be mechanically restrained for the last three sections at the end of a run when typing into existing box culvert or a structure. The joint on either side of a new structure shall also be mechanically restrained.

610.1.03 - BUILDING LATERAL SEWERS AND STORM WATER DRAINS

A. GENERAL

The size, type of material, location, and direction of existing building laterals and the approximate distances from the nearest existing downstream manhole are shown on the plans and on the TV inspection logs, available for inspection at the City Engineering Division upon request. The Contractor shall be responsible for locating and verifying the size and type of material of each existing building lateral in the field. This shall include dye testing or electronic locating methods where necessary. This shall be incidental to the work and no additional compensation will be made for the location process or delays caused by this verification.

Make all lateral reconnections in accordance with the details shown in the plans. Materials to be used for this work shall be as specified in Chapter 3.4.0 of the Standard Specs, amended as follows: The material to be used shall be of equal size of the existing lateral and of the same material as the relayed mainline sewer unless otherwise specified on the plans.

Adaptors, couplings, and connectors shall be watertight and as shown on the plans, or shall be approved by the Engineer. Joints shall be rubber gasket as approved by

the State and local plumbing code. Cement mortar or glued joints are not acceptable.

Excavation and backfill shall conform to the applicable requirements of Chapter 3.4.0 of the Standard Specs and as herein modified. The maximum trench width shall be the outside diameter of the pipe plus 24 inches. Bedding shall be the same as required for the mainline. Backfill in the pipe zone shall be the same as required for mainline repair.

Install lateral reconnections in accordance with the applicable requirements of Chapter 3.4.0 of the Standard Specs. Use factory fabricated wyes or tees without glued or fused pieces. Provide bends, suitable lengths of straight pipe, and joints for dissimilar pipe as required. The minimum slope of the lateral reconnection shall be 1/4 inch per foot. Sanitary lateral reconnections which are to be extended shall be laid at normal depth for a new lateral with the adjustment to the existing grade of the lateral being made beginning at 5 feet from the back of curb or where directed by the Engineer.

B. LATERAL AND SUMP PUMP COLLECTOR SYSTEM TRACER WIRE WITH ACCESS BOX

This section shall only be applicable where shown on the plans or directed by the Engineer, and shall be incidental to the cost of lateral installation.

1. DESCRIPTION

When stated on the plans or directed by the Engineer, building sewer laterals and sump pump collector systems shall be installed with a tracer wire in accordance with the State of Wisconsin Administrative Code Chapter SPS 382.30(11)(h). This code requires that all new, non-metallic building sewers (including sanitary, storm, sump pump collector systems and private sanitary sewers) and water services installed must be accompanied by a means of locating the underground pipe.

- a. A pipe locator conductor (tracer wire) shall be installed on all non-metallic (PVC, PE, clay, concrete and other non-metallic) sewer laterals and sump pump collector systems within the limits of the project as noted on the plan or directed by the Engineer. The conductor shall be placed along the top of the sewer lateral pipe from the sewer main or structure up to the property line or the end of the installation beyond the roadway as directed by the Engineer. On sump pump collector systems, the conductor shall run from the storm structure to the clean-out and from clean-out to clean-out or as directed by the Engineer.

Wrapping of the tracer wire on the pipe is prohibited. The conductor shall be held in place with ties or hitches spaced no more than 10 feet apart. The ties or hitches shall be spaced no more than 10 feet apart. The conductor shall be a minimum of 12-gauge standard solid copper wire with a green PVC or 30 to 45 MIL of Polyethylene coating to prevent corrosion. The wire shall be rated for buried and wet conditions. The conductor itself will be one continuous loop with the two wire ends connected to the tracer wire access box.

- b. The tracer wire shall be brought to the surface at the property line, end of the lateral installed, at each sump pump collector system clean-out, or at a location directed by the Engineer within a covered access device. The covered access device (tracer wire access box) may be a terminal box, valve box, a small diameter PVC conduit or a cleanout. Within the covered access device, the Contractor shall provide an extra 18 inches of wire. The lid of the covered access device shall have "SEWER" permanently engraved on it by the manufacturer. The lids shall be cast iron accompanied with connection holes where the Contractor shall connect the tracer wire with stainless steel terminal bolts. The lid shall be bolted with a standard pentagonal head key.
- c. Please be aware that below grade splices are prohibited.
- d. Each tracer wire shall be field tested after installation is complete.
- e. The Valvco Tracer Wire Access Box (<http://www.cptest.com>) and the Bingham & Taylor Cathodic Test Box (<http://www.binghamandtaylor.com/cathodic.htm>) are considered acceptable devices for this specification.

To minimize damage to the tree's root zone during the installation of the sanitary sewer lateral installation, no excavation shall be made within the following limits:

<u>Tree Diameter (In.) (@ 4.5' Above Ground)</u>	<u>No Excavation Limits Distance (ft.) from Trunk</u>
0 - 2	1
3 - 4	2
5 - 9	5
10 - 14	10
15 - 19	12
Over 19	15

The Contractor may encroach on the above limits if the sanitary sewer lateral to which the lateral piping will be connected or the house side of the existing City sidewalk is within the specified no excavation zone. The Contractor shall keep these

excavations as small as possible and shall contact the Engineer at least 3 days prior to starting the installation so they may notify the City Forester.

610.1.04 - MANHOLES

The Contractor shall be responsible for cleaning all sewer and water structures in the project area of all debris at their own expense.

A. INVERTS

Benches on all manholes shall be constructed at a minimum up to the crown of each pipe and sloped as specified for a sanitary or storm manhole as needed. Refer to File No.s 11 and 12 of the Standard Specs.

B. CASTINGS

New frames and covers are required on all new manholes, and new frames, grates, and back boxes are required for new inlet structures unless otherwise noted on the plans, and shall be supplied by the Contractor unless otherwise directed in the contract or by the Engineer. All castings shall be considered incidental to the applicable structure item unless otherwise stated as separate bid items.

Sanitary sewer manhole covers shall be self-sealing with an o-ring gasket and of a non-modernized design. The cover shall weigh approximately 143 pounds. They shall be Neenah R-1661-B or equal. They shall have two concealed pick holes. **The City of Wauwatosa will furnish these self-sealing covers with the new frame and grate unless specified otherwise in the plans. The Contractor shall pick them up at the Municipal Public Works Building at 11100 W. Walnut Road and install them. The contractor shall call 414-471-8422 a minimum of 1 day prior to picking up the materials. The Contractor shall provide all labor & equipment necessary to load the materials and deliver them from the DPW Yard to the jobsite.**

Storm sewer manhole covers shall be as shown in the plan details or equal and of a non-modernized design. The cover shall weigh approximately 152 pounds. Single and double storm sewer inlets shall be as shown in the plan details or equal and of a non-modernized design.

Frames for sanitary and storm sewer MHs shall be compatible with the covers and also be of a non-modernized design. Frames shall weigh approximately 369 pounds.

All manhole frames, iron rings, covers, storm water inlet or catch basin frames, grates, and back boxes which are removed from existing structures and are not reused shall remain the property of the City. The Contractor shall deliver these to the Municipal Public Works Building at 11100 West Walnut Road, Wauwatosa, WI.

C. FRAME/CHIMNEY JOINTS AND SEALS

Unless the manholes are to be adjusted and set to grade under a separate contract, the frame/chimney joints shall be as specified in the Standard Specs. Sanitary sewer manholes shall be constructed with a Type I, flexible watertight frame/chimney joint as detailed in Section 3.5.4(f)1.(a) of the Standard Specs. Seals shall be incidental to any manhole work unless otherwise stated as a separate bid item.

The Engineer approved manhole frame/chimney seal, where required, shall consist of a flexible rubber sleeve, overlapping extension or extensions as needed, and stainless steel bands, and shall extend from the frame to the cone of the new manhole to insure the chimney is fully covered. They shall be furnished and installed by the Contractor and shall be an internal rubber sleeve as manufactured by Cretex Specialty Products, N16 W23390 Stoneridge Drive, Suite A, Waukesha, Wisconsin, 53188, NPC, Inc., 250 Elm Street, PO Box 301, Milford, NH 03055 or Engineer approved equal. The Contractor shall use the proper tools for installation of the seals.

If it appears a flexible rubber seal will not fit or function properly in a manhole, and the Contractor has permission from the Engineer, a mastic seal or equal may be spread on the chimney in lieu of installing the rubber one. The Engineer must be present at the time of installation to verify all chimney joints were thoroughly covered.

1. SURFACE PREPARATION

Surface preparation shall be as follows or as recommended by the manufacturer if their requirements are more stringent:

- a. Remove manhole cover and allow any accumulated fumes to dissipate, open additional manholes or use blower to ventilate, if necessary.
- b. Power wire brush the lower 3 inches of the manhole frame to remove any loose rust or scale and repair any imperfections by either grinding smooth or filling with mortar. A reasonably smooth, clean sealing surface is required.
- c. Realign the casting if it is offset more than approximately 2 inches from the chimney.
- d. Make a visible line or series of alignment marks around the frame 2-3/4 inches up from the bottom edge of the frame for normal positioning. The sleeve can be installed higher in the frame if necessary, in which case the marks should be raised accordingly.

- e. Provide a 4 inch wide sealing surface on the manhole cone deck (i.e. not on the adjusting rings). Remove all loose and protruding mortar and brick as needed to provide a sealing surface.
- f. All sealing surfaces must be circular, reasonably smooth, clean and free of any loose material or excessive voids. If such a surface does not exist for the bottom of the sleeve to seal against, the Contractor shall prepare one. The Contractor shall use one-component, quick-set, high-strength, non-shrink, polymer modified patching mortar which has been formulated for vertical or overhead use to prepare the uniform vertical sealing surface.
- g. If the bottom of the sleeve is to seal against the top of an eccentric (straight side) cone and an inadequately high vertical surface does not exist, the contractor shall contact the manufacturer to obtain details for building the required vertical surface.
- h. If any caulk is used to fill minor irregularities in the bottom sealing surface, the caulk shall be a butyl rubber caulk conforming to AASHTO M-198, type B. When used, the Contractor shall apply a single bead of the caulk to the center portion of the lower sealing surface of the sleeve. The Contractor shall not use any other type of caulking material. Caulk is considered incidental to the seal installation and the Contractor will not be paid extra for it.

2. CRETEX INTERNAL MANHOLE FRAME CHIMNEY SEAL INSTALLATION

The Contractor shall also refer to the manufacturer's literature for additional installation variations and options.

- a. Install the rubber sleeve with the printing at the top and line the top edge up with the previously applied marks. Any flaws in the manhole frame, such as minor cracks, pits or protrusions, shall be repaired by either filling with mortar or grinding smooth.
- b. Lightly lubricate the outside of one stainless steel band with gasket lube and install it in the lower band recess so that the slotted end laps over the end with the studs and the studs extend through the adjustment slot. Put on the self-locking nuts and tighten sufficiently to draw the lapped ends of the band close enough to allow the attachment of the expansion tool. Position the expansion tool and expand the band as required to provide a watertight seal and tighten the two lock nuts.
- c. Conduct a water leakage test on the lower band under the supervision of the Engineer. The Engineer shall determine how much water should be used for a proper test and shall decide if the seal passes or fails. If it fails, the Contractor may choose to re-install the lower band and repeat the test, or apply a bead of butyl rubber caulk, conforming to AASHTO M-198, Type B, to

the lower sealing surface of the sleeve to fill any minor irregularities in the masonry surface to the satisfaction of the Engineer present.

- d. Lubricate the second band and install it in the upper recess, attach the tool and expand as before, keeping the bands parallel and a minimum of approximately 3 inches apart. The bands can be put closer together if excessive sleeve expansion is specifically required.
- e. Check the top and bottom edges of the installed sleeve to insure that it has been properly compressed and sealed against the two surfaces.
- f. **The Engineer will not pay the Contractor for any internal manhole seals unless the Engineer has witnessed a passing water leakage test or witnessed satisfactory application of butyl rubber caulk or mastic.**

D. FRAME ADJUSTMENTS & MASONRY

The masonry mortar and concrete bricks shall comply with the requirements of Section 519 of the State Specs and shall be incidental to the work.

Adjustments on manhole frames must be done after the asphalt base/binder has been laid and before the surface course is laid, and shall match 1/4 inch ("string bounce") below the surface grade. Backfilling around the frames after adjustment shall be done with compacted fill as specified for the pavement base, and compacted asphalt base/binder material. Adjustments shall be incidental to the work unless otherwise specified as a separate bid item, and any adjustment bid item shall include all the labor, equipment, and materials needed.

The minimum dimensions for the pavement box-outs to perform adjustments shall be large enough to fully accommodate compaction by mechanical means. **The use non-mechanical means will not be permitted for compacting the lower layers around manholes without the express written approval of the Engineer.**

While performing the masonry work involved in making adjustments, the Contractor shall provide the means to intercept dropped materials before they reach the bottom of the structure, and shall clean the structure of any such materials at the bottom before final payment will be made. This shall be incidental to the work.

New sewer structures shall be built within approximately 4 inches of grade needed for the frame, requiring final frame setting during adjustments. Sewer structures to be in concrete pavement, at the time the surrounding concrete pavement is poured, shall have frames that are "wedged" high enough during concrete paving that the aggregates in the agitated concrete mix can move freely under the frame, and thus allow the frame to sit on solid concrete.

When additional masonry replacement is required to an extent which includes a normal step location, a new step must be incorporated as part of the work under that item. Replacement of masonry in poor condition is required even if it extends farther than listed on the plans. Sanitary manhole masonry work must be performed before the installation of internal seals or Pro Rings.

E. PRECAST MANHOLE JOINTS

All joints between sanitary manhole sections including base, riser(s), and cone shall be sealed with a high-strength external perimeter sealing band, consisting of a heavy polypropylene backing, rubberized mastic seal, woven polypropylene reinforcing, and heavy-duty steel straps, under the supervision of the Engineer. The external seal shall allow the manhole structure to pass the ASTM C-1244 vacuum test as described in Chapter 3.7.6 of the Standard Specs (see section 610.1.07 C of the City Specs for internal chimney seals). External perimeter sealing band shall be Mar Mac MacWrap or approved equal. Vacuum test shall be performed after all seals are in place, under the supervision of the Engineer, who will decide if it passes.

For sanitary manholes which have an outside drop, an Engineer approved mastic seal shall be used in lieu of Mar Mac MacWrap for all joints which are non-circular around the full perimeter of the manhole at the applicable joint(s). This mastic seal shall also be installed on other non-circular joints at the direction of the Engineer.

All external joint sealing shall be incidental to the cost of the manhole, regardless of sealing method, unless otherwise noted as a separate bid item.

F. BYPASS PUMPING

Contractor shall submit bypass pumping plans for review by the Engineer at least 3 business days prior to the work. A bypass pumping plan is required for ALL bypass pumping that occurs. The Contractor shall notify the Engineer 24 hours prior to commencement of the bypass pumping operation. The Contractor's plan for bypass pumping shall be approved by the Engineer before the Contractor will be allowed to start bypass pumping. This shall be incidental to the utility work.

Bypass Pumping plans shall include but is not limited to all of the information below:

- Locations of the MH where pumping will occur and the discharge MH
- Pump(s) size and flow capacity
- Duration of bypass pumping
- For proposed 24 hr pump operations, provide the following additional information:
 - Back-up system information in event of pump failure

- 24 hour emergency contact

G. MANHOLE CONNECTIONS

Where a new manhole is to connect to an existing sewer that will not be relayed at a later point in the project, up to the first 6 linear feet of pipe used in this connection shall be included in the price of the new manhole and considered incidental to the work.

If the existing pipe is found to be in poor condition, the Contractor shall inspect it to find how much farther they must dig to expose a section in at least acceptable condition. If the length is reasonably short and/or at the Engineer's direction, the Contractor shall be paid for each linear foot of pipe used beyond the initial 6 feet of connection. If the next acceptable pipe is unreasonably far away, the Contractor shall seek instructions from the Engineer for how to proceed.

610.1.05 - CATCH BASINS, STORM WATER INLETS & INLET MANHOLES, AND STORM WATER DRAINS

Storm water inlets and inlet MHs shall be constructed in conformance with File No. 11, 12, and 28 of the Standard Specs modified so as to accommodate the required frame (refer to the standard details and special provisions on the plan), and shall include a 6 inch stub for future lateral connection and a 4 or 6 inch stub for future underdrain connection at locations and directions as shown on the plans or as directed by the Engineer. The cost of these stubs shall be included in the unit price bid per inlet. These stubs shall be made of PVC SDR35 pipe.

The frame, grate, and curb box shall be furnished by the Contractor. The specific casting to be used on the job will be noted on the plans and listed in the special provisions of each individual contract. Castings shall be incidental to the structure unless otherwise noted as a separate bid item.

Storm water inlet manholes shall have a poured bench meeting the requirements of a standard manhole. Where the plans call for the construction of a storm water inlet rather than a manhole or inlet manhole on a storm sewer, the inlet shall also have a paved invert as specified for a standard MH.

Where the depth of the manhole is too shallow to accommodate the standard cone top section, a flat top slab shall be substituted for the cone and shown in the shop drawings. The steps and cover shall be to the side identified on the plans as the straight side. No additional compensation will be given for this change. Payment will be at the per vertical foot bid and contract price.

Underdrains, to be installed where indicated on the plans, shall be 6 inch perforated PVC wrapped in geotextile fabric (a "sock"), and be in accordance with section 612 of

the State Specs. Bedding of 3/8 inch limestone chips shall be used around the entire pipe and be in accordance with Table 32 of section 8.43.2 of the Standard Specs unless otherwise specified.

610.1.06 - ACCEPTANCE AND QUALITY CONTROL OF SANITARY AND STORM SEWERS

A. FINAL SEWER CLEANING

Prior to final acceptance of the sewer system by the Engineer, flush and clean all parts of the system. Remove all accumulated construction debris, rocks, gravel, sand, silt, and other foreign material from the sewer system at or near the closest downstream manhole. If necessary, use water jet, mechanical rodding or bucketing equipment. If any foreign matter is still present in the system upon final televised inspection by the Contractor, re-flush and clean the sections and portions of the lines as required.

The Contractor shall also submit a written report of the sanitary sewer cleaning. This report shall identify the sewer segments cleaned and the type and volume of debris removed from the sanitary sewers.

Perform a mandrel test and provide the results to the Engineer as part of the acceptance process.

B. ACCEPTANCE OF SANITARY AND STORM SEWER RELAYS BY CLOSED CIRCUIT TV VIDEO INSPECTION

Prior to final acceptance of any sanitary sewer or storm sewer spot relay, the Contractor shall inspect by means of remote closed circuit television equipment the entire segment of sanitary sewer, manhole-to-manhole, on which the repair was made, not just the short length repaired out of the manhole. It is the City's intent to have all manhole-to-manhole sewer spans disturbed, replaced, or repaired as part of the contract, to be internally inspected. Sewers shall be cleaned prior to inspection and all manhole connections shall be shown. A USB external hard drive of the inspections shall be furnished to the Engineer for review and acceptance. Televising shall be incidental to the sewer work.

The following conditions shall apply to the sewer acceptance TV inspection:

1. CCTV operators shall be NASSCO trained and certified in the use of Pipeline Assessment and Certification processes and nomenclature.
2. Video shall be furnished on an external USB hard drive that will not be returned to the Contractor.

3. The TV camera shall travel through the sewer at a maximum rate of 30 feet per minute.
4. The camera shall stop at the beginning and end joint of each relay for a 10 second period.
5. The camera shall travel in the downstream in all cases.
6. The lens of the camera shall be cleaned at each MH or when directed by the Engineer.
7. The videos shall have an on-screen display showing, at a minimum, the following:
 - a. Upstream and downstream MH numbers
 - b. Footage from upstream MH
 - c. Date of inspection
8. Sewers shall not be televised within 48 hours of a rainfall event greater than 1/4 inch.
9. Jetting of pipe relay segments shall be completed no less than 30 minutes prior to televising and under normal functioning conditions.

This inspection shall be made as soon as practicable after the backfill has been consolidated. If the Contractor chooses to wait until after paving or restoring the trench surface to televise the sewer, they will be solely responsible for any costs incurred from any potential repairs required to make the work acceptable, including, but not limited to, additional restoration and/or paving.

Reasons for rejection of the relay will include but not be limited to:

- Dropped joint
- Broken joint
- Open or offset joint
- Sag in repair
- Deflected pipe
- Leaks

The Engineer shall determine if any of these conditions exist and if they are excessive enough to be considered defective and warrant replacement. If directed by the Engineer, the Contractor shall promptly, at their expense, correct all defects.

C. MANHOLE VACUUM TESTING

Contractor shall vacuum test sanitary manholes in accordance with the applicable requirements of Chapter 3.7.6 of the Standard Specs under the supervision of the Engineer. It is highly recommended that the Contractor test sanitary sewer manholes immediately after installation and prior to backfilling. If the Contractor chooses to wait until after paving or restoring the trench surface to vacuum test the manholes, they will be solely responsible for any costs incurred from any potential repairs required to make the work acceptable, including, but not limited to, additional restoration and/or paving.

Plug lift holes with non-shrink grout. If a manhole fails the initial test, make necessary repairs with non-shrink grout or other acceptable and approved materials. The Contractor shall continue re-testing until the Engineer determines a satisfactory test is obtained. All testing shall be incidental to the contract.

D. SEWER TRENCH AND MANHOLE EXCAVATION DYE WATER FLOOD TEST

The City of Wauwatosa, at its discretion, may perform dye water flood testing on all sewer trenches and manhole excavations prior to final surface restoration or final payment to identify infiltration into the system. Dye water flood testing of sewer pipe and manholes shall be in accordance with the applicable requirements of Chapter 3.7.2 Water Infiltration Test of the Standard Specs. Infiltration rates as identified in the Standard Specs shall determine pass or failure of the pipe and manholes. If a pipe or manhole fails the initial test, the Contractor shall make necessary repairs at their own expense with approved materials. The Engineer may continue re-testing until a satisfactory test is obtained.

E. TRENCHLESS UTILITY INSTALLATION NEAR CITY SEWER FACILITIES

The City of Wauwatosa, at its discretion, may require contractors installing utilities using a trenchless method (such as directional boring) within the vicinity of existing storm sewer and existing sanitary sewers, including storm sewer and sanitary sewer laterals, to televise these facilities before and after completing work to ensure that the facilities were not damaged during installation of a trenchless utility. This includes, but is not limited to installation of gas mains and services, electrical lines and services, sewers and laterals, telephone lines, fiber optics, and water mains and services.

Televising requirements shall follow Paragraph B. of this section.